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Manuscript title: Relationship between pore size distribution and compressibility of a lateritic soil

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Abstract

The aim of this paper is the evaluation of the relationship between pore size distribution and compressibility of a lateritic soil. Soil fabric was analysed under different conditions of preparation: compacted, reconstituted, and intact, by means of Mercury Intrusion Porosimetry tests upon loading to different stress levels. Results show that all samples present bimodal pore distribution. However, the conditions of preparation and loading affected macropores more intensely, with subtle effects on micropores. This behaviour was attributed to the presence of aggregations with cementation. The compression curve of macro and micropores were determined and it was observed that the compression curve of macropores is very similar to the compression curve of the samples. In addition, it was observed that the fabric parameter regarding the uniformity of macro and micropores changed during loading. The new fabric parameter proposed captures the change in pore uniformity with loading. This parameter appears to be related to the soil compressibility, indicating that the most compressible samples also show greater changes in the uniformity of macropores.

Keywords: Compressibility; Fabric of soils; Lateritic soils

1. Introduction

Geotechnical studies carried out over the years have shown that there is a relationship between the mechanical behaviour of soils and their microstructure, mainly in terms of compressibility (Alonso, Pinyol & Gens, 2013; Lambe & Whitman, 1969; Delage *et al.*, 2006; Koliqi, Vulliet & Laloui, 2010; Ng, Mu & Zhou, 2017; Yu, Chow & Wang, 2016; Ng, Akinniyi & Zhou, 2020; Kochmanová & Tanaka, 2011; Sivakumar & Wheeler, 2000; Ng *et al.*, 2020; Wang *et al.*, 2020). The fabric effect on compressibility can be observed by carrying out tests on samples prepared under different initial conditions, such as: intact, compacted and reconstituted (Burland, 1990; Leroueil & Vaughan, 1990), where changes in behaviour are observed in terms of compression indexes and preconsolidation stresses. Microstructural evaluation based on Mercury Intrusion Porosimetry (MIP) and Scanning Electron Microscopy (SEM) tests has been used to measure soil fabric quantitatively and qualitatively. Soil fabric can assume pore size distributions that vary from mono-modal to multi-modal (more than two dominant pore sizes), however mono-modal and bimodal (two dominant pore sizes) distributions are the most widely reported (Kühn *et al.*, 2021; Romero, 2013; Pedrotti & Tarantino, 2014; Miguel & Bonder, 2012; Delage *et al.*, 1996). Highly weathered lateritic tropical soils present bimodal pore size distributions for both intact and compacted samples (Santos & Esquivel, 2018; Otálvaro, Cordão-Neto & Caicedo, 2015; Cordão Neto *et al.*, 2018). These soils are distinguished by their mineralogical composition, with marked presence of iron and aluminium oxides and hydroxides (Otálvaro *et al.*, 2016; Mitchell & Soga, 2005; Miguel & Bonder, 2012) which contributes to the formation of aggregations in their structure (Camapum de Carvalho *et al.*, 2006).

Despite many studies analysing jointly the distribution of pores and the mechanical behaviour of soils, there is no constitutive relationship between mechanical and porosimetry parameters. In this sense, the aim of this paper is the evaluation of the relationship between pore size distribution and compressibility of a lateritic soil. For this, a series of MIP tests were performed on samples prepared under different conditions, namely: Intact, Compacted, and Reconstituted, and loaded unidimensionally to different stress levels. Using a fitting equation, the void ratio and the compression index of macro- and micropores were separated, and a new fabric parameter was proposed. In this way, this paper presents a new approach to analyse porosimetry curves and obtain compressibility parameters related to macropores that are suitable for lateritic soils.

2. Materials and methods

The soil used in this research is a residual weathered soil reddish in colour with characteristics of lateritic soils. XRD tests on these samples show a predominance of gibbsite and quartz minerals, and to a lesser extent, kaolinite and hematite (Guimarães, 2002). This material presents liquid limit, w_L , of 42%, plastic limit, w_P , of 24%, plasticity index, PI, of 18%, and specific gravity, G_s , of 2.748. It is classified as a low plasticity clay (CL) according to Unified Soil Classification System (ASTM, 2017). For this study, samples were collected at a depth of 1.7 to 2.3m and prepared under three different conditions: intact (I), compacted (C) and reconstituted (R) (**Figure 1a**).

Figure 1b shows the particle size distribution of tests performed with and without dispersant. Particle size distribution of sandy clay is observed in tests carried out with dispersant while silty sand distribution is observed in tests without dispersant. This difference is indicative of soils with aggregations formed by the cementation of smaller particles, which are stable in the presence of water but can be broken with dispersant (Camapum de Carvalho *et al.*, 2006; Ng, Akinniyi & Zhou, 2020; Otálvaro, Cordão-Neto & Caicedo, 2015).

The compressibility of the samples was assessed by means of oedometric tests following D2435 standard (ASTM, 2020). Specimens (I, C and R) of approximately 1 cm³ were moulded from freshly prepared ($\sigma'_v = 0$ kPa) samples and from samples that underwent vertical stresses of 50, 100, 200, 400 and 800 kPa. These specimens were subjected to freeze-drying and their pore size distributions were evaluated by means of Mercury Intrusion Porosimetry (MIP) tests using Autopore IV 9500 Micromeritics equipment.

It is important to note that the MIP technique has limitations (Romero & Simms, 2008), which make the void ratio obtained in this test different from the void ratio of the sample.

Table 1 presents the final void ratios intruded by mercury (e_{MIP}) and the void ratio of the samples (e_{sample}) for each stress level considered. This difference may have an impact on the fabric parameters as discussed later in this paper.

A fitting equation as proposed by Durner (1994) and Lopes, Cordão-Neto & Tarantino (2014), based on van Genuchten (1980)'s equation, was used to fit the MIP intrusion data.

$$e_{MIP} = e_{macro} \left[\frac{1}{1 + (\alpha_{macro} \cdot D)^{n_{macro}}} \right]^{1 - \frac{1}{n_{macro}}} + e_{micro} \left[\frac{1}{1 + (\alpha_{micro} \cdot D)^{n_{micro}}} \right]^{1 - \frac{1}{n_{micro}}} \quad (1)$$

where: D is the pore diameter intruded, μm , e_{macro} and e_{micro} are the void ratios of macro and micropores respectively, α_{macro} and α_{micro} are the fitting parameters related to the size of dominant macro and micropores respectively, μm^{-1} , and n_{macro} and n_{micro} (dimensionless) are the fitting parameters related to the slope of the macro and micropores intrusion curves, respectively.

The separation between macro and micropores was performed by best fitting of Eq. 1 which is highlighted in **Figure 3**. The R^2 of the fittings ranged from 0.98 to 1.00.

3. Results

3.1 Compressibility

Oedometric compression tests are presented in **Figure 2** while compressibility parameters (preconsolidation stress, σ'_{pc} , compression index, C_c , and recompression index, C_r) are presented in **Table 2**. It can be observed that the sample preparation conditions strongly affected the compressibility. The intact sample shows the highest compressibility due to its more porous structure. On the other hand, compacted and reconstituted samples present lower compressibility, as compaction or mixing at high water content during preparation stage caused structural alteration on those samples. In general, it is noticed that the initial structure has a relevant role in the compressibility of the samples.

3.2 Pore size distribution

Figure 3a, c, e show the cumulative intrusion curves of the samples (I, R and C) and **Figure 3b, d, f** present their respective pore size distributions. All samples show a bimodal behaviour for all loading stages investigated, with loading having the greatest impact on macropores. Without loading, the intact sample has larger macropores in respect to compacted and reconstituted samples, which is typical of collapsible soils (Camapum de Carvalho *et al.*, 2006). This shows that preparation conditions have strong impact on the pore size distribution and its evolution with loading, particularly decreasing the volume and dominant diameter of the macropores (**Table 1**). The limiting diameter between macro and micropores is different from sample to sample and according to loading, as shown in **Figure 3**.

4. Evolution of fabric parameters with loading

The compression curves of the macro and micropores were determined based on macro and micro void ratios obtained through MIP tests for the different applied stresses. **Figure 4** shows the oedometric, macro and micropores compression curves, where the compression index of the virgin portion of the oedometric test (C_c - dimensionless), as well as the compression index of the macro ($C_{c_{macro}}$ - dimensionless) and micropores ($C_{c_{micro}}$ - dimensionless) are highlighted. These values are also presented in **Table 2**. The shape of the compression curve of the macropores is very similar to the compression curve of the samples, that is, the compressibility of the macropores predominates over the micropores, as observed by the $C_{c_{macro}}$ values in **Table 2**.

The largest variations in macropores occur after the preconsolidation stress for intact and compacted samples. This observation leads to the conclusion that plastic deformations are mostly associated with deformations of the macropores. It is then reasonable to say that the process by which the largest pores close is irreversible. However, in the reconstituted samples, the trend of reduction of the macropores, as well as the compressibility curve of the sample are almost constant, with no occurrence of preconsolidation stress. Meanwhile, the void ratio of the micropores shows little variation due to loading, which may be related to the nature of the aggregations with smaller and cemented particles. Thus, it is possible to assume that the compressibility of the samples is largely related to the deformation of the macropores while the micropores remain mostly unaffected.

Delage (2010) found that the pore uniformity, given by the slope of the mercury intrusion curve (C_p index), has a good correlation with the compressibility (C_c) of some unimodal soils. The analyses performed by Delage (2010) were adapted in this work to bimodal soils. Thus, two coefficients were established: $C_{p_{macro}}$ and $C_{p_{micro}}$ for macro and micropores, respectively. **Figure 5** shows the evolution of the parameters for intact samples under 3 different stress levels. There is a change in the parameters, with a significant decrease in $C_{p_{macro}}$, indicating that the macropores are becoming well graded. The same behaviour in terms of macropores is seen in the other samples (see $C_{p_{macro}}$ in **Table 1**). On the other hand, $C_{p_{micro}}$ shows decreasing uniformity (**Figure 3** and **Table 1**). Thus, the behaviour of the macropores appears to be more significant than the modifications observed for the micropores.

Figure 6 shows the changes in parameter $C_{p_{macro}}$ for each level of vertical stress, together with the compression curves of the sample and macropores. Since the dominant macropores change with loading, parameter C_p shows a different behaviour from that observed by Delage (2010) for St Marcel Clay. In this sense, it is necessary to establish a new index that can represent the change in the uniformity of each pore size for a given increase in stress, in which:

$$Cuc_{macro} = \frac{\Delta C_{p_{macro}}}{\Delta \text{Log} \sigma'_v} \quad (2)$$

$$Cuc_{micro} = \frac{\Delta C_{p_{micro}}}{\Delta \text{Log} \sigma'_v} \quad (3)$$

where: $C_{p_{macro}}$ and $C_{p_{micro}}$ (dimensionless) are the slope of the intrusion curve in the region of macro and micropores respectively, i.e., $C_p = \Delta e_{MIP} / \Delta \text{Log} D$; σ'_v is the effective stress, kPa; and Cuc_{macro} and Cuc_{micro} (dimensionless) are the macro and micropores index of uniformity

change, respectively. Cuc index is an indirect measure of how the compression coefficient varies with pore diameter, as follows:

$$Cuc_{macro} = \frac{\Delta C p_{macro}}{\Delta \text{Log} \sigma'_v} \quad (4)$$

$$Cuc_{macro} = \frac{(C p_{macro})_i - (C p_{macro})_j}{\Delta \text{Log} \sigma'_v} \quad (5)$$

$$Cuc_{macro} = \frac{(\Delta e_{macro} / \Delta \text{Log} D)_i - (\Delta e_{macro} / \Delta \text{Log} D)_j}{\Delta \text{Log} \sigma'_v} \quad (6)$$

If $\Delta \text{Log} D$ intervals are the same, then:

$$Cuc_{macro} = \frac{(\Delta e_{macro} / \Delta \text{Log} \sigma'_v)_i - (\Delta e_{macro} / \Delta \text{Log} \sigma'_v)_j}{\Delta \text{Log} D} \quad (7)$$

$$Cuc_{macro} = \frac{(C c_{macro})_i - (C c_{macro})_j}{\Delta \text{Log} D} \quad (8)$$

$$Cuc_{macro} = \frac{\Delta C c_{macro}}{\Delta \text{Log} D} \quad (9)$$

where: i and j represent same state (I, R or C) samples that underwent MIP tests at different loading stages.

Values of Cuc_{macro} and Cuc_{micro} are presented in **Table 2**. These were calculated between Cp at stresses of 400 and 800 kPa, as this is the stress range used to derive Cc . Higher values of Cuc_{macro} indicate that the pores of the macrostructure change easily and thus they can be related to compressible soils (higher Cc_{macro}). In contrast, smaller Cuc_{macro} values indicate less compressible soils (lower Cc_{macro}). Cuc_{micro} , on the other hand, presents negative values, indicating an increase in uniformity. It is important to note that the relationship between compressibility (Cc) and fabric parameters (Cc_{macro} and Cuc_{macro}) will be as good as the relationship between the void ratio of the sample (e) and the MIP void ratio (e_{MIP}). In this sense, it is not a surprise that the relationship between the fabric macro parameters and the sample compressibility of the compacted and reconstituted samples are better than that of the intact sample (**Table 1** and **Figure 7**). Excluding the intact sample, the correlation between Cc and Cuc_{macro} is satisfactory (**Figure 7**, $R^2 = 0.91$ for the $y = x$ regression line). The introduction of macro and micro Cuc parameters do not make the use of Cc_{macro} and Cc_{micro} inappropriate. However, as previously discussed, Cuc has interesting physical meanings that could be further explored.

5. Conclusions

This paper evaluated the fabric modifications of a lateritic soil under different conditions of preparation (Intact, Reconstituted and Compacted). From the tests carried out, relationships were established between the compressibility parameters commonly used in geotechnics, such as: the compression index and preconsolidation stress, and fabric parameters of the soil. In this sense, new parameters have been proposed: (1) Cc_{macro} and Cc_{micro} that represent the compressibility of macro and micropores respectively, (2) Cp_{macro} and Cp_{micro} that represent the uniformity of macro and micropores respectively, (3) Cuc_{macro} and Cuc_{micro} that represent the change in the uniformity of macro and micropores, respectively.

The following points can be highlighted in respect to the soil under study:

- (1) The compressibility of the sample is dominated by the compressibility of the macrostructure, that is, Cc_{macro} is very similar to Cc for compacted and reconstituted

samples. For the intact sample the difference between e_{MIP} and e results in the discrepancy observed between C_c and $C_{c_{macro}}$. The contribution of $C_{c_{micro}}$ towards the compressibility of the sample is irrelevant.

- (2) The uniformity of the pores changes during loading. However, the uniformity of the macropores ($C_{p_{macro}}$) is more affected by loading than the uniformity of the micropores ($C_{p_{micro}}$).
- (3) Samples with higher macropores index of uniformity change ($C_{uc_{macro}}$) also present greater compressibility (C_c) and, consequently, higher compressibility of the macrostructure ($C_{c_{macro}}$).

Further studies with a wider range of soils may contribute to the verification of the correlations between the parameters discussed here.

Acknowledgements

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List of notations

σ'_{pc}	preconsolidation stress
σ'_v	vertical effective stress
I	intact sample
R	reconstituted sample
C	compacted sample
C_c	sample compression index
$C_{c_{macro}}$	macropores compression index
$C_{c_{micro}}$	micropores compression index
C_p	sample mercury intrusion curve slope angle
$C_{p_{macro}}$	macropores mercury intrusion curve slope angle
$C_{p_{micro}}$	micropores mercury intrusion curve slope angle
$C_{uc_{macro}}$	macropore index of uniformity change
$C_{uc_{micro}}$	micropore index of uniformity change
C_r	recompression index
w_L	liquid limit
w_P	plastic limit
PI	plastic index
G_s	specific gravity
CL	low plasticity clay according to Unified Soil Classification System
e_{sample}	sample void ratio
e_{MIP}	final MIP void ratio
D	diameter of intruded pore

$D_{d_{macro}}$	dominant macropore diameter
$D_{d_{micro}}$	dominant micropore diameter
α_{macro}	fitting parameter related to the size of dominant macropore
α_{micro}	fitting parameter related to the size of dominant micropore
n_{macro}	fitting parameter related to the slope of the macropore intrusion curve
n_{micro}	fitting parameter related to the slope of the micropore intrusion curve

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Table 1. Sample data: e_{sample} , e_{MIP} , Dd_{macro} , Dd_{micro} , Cp_{macro} , Cp_{micro}

Sample	Consolidation		MIP				
	σ'_v (kPa)	e_{sample}	e_{MIP}	Dd_{macro}	Dd_{micro}	Cp_{macro}	Cp_{micro}
I	0	1.733	1.455	38.630	0.033	1.154	0.398
	50	1.636	1.255	17.083	0.027	0.600	0.552
	100	1.483	1.053	6.977	0.025	0.423	0.719
	200	1.286	1.060	6.771	0.026	0.478	0.668
	400	1.094	0.939	3.618	0.025	0.344	0.734
	800	0.887	0.820	1.972	0.023	0.240	0.777
C	0	1.149	1.065	24.933	0.031	0.834	0.490
	50	1.073	0.989	22.348	0.028	0.635	0.589
	100	1.049	1.011	15.465	0.030	0.520	0.528
	200	0.995	0.931	9.497	0.026	0.408	0.644
	400	0.896	0.941	8.513	0.028	0.394	0.594
	800	0.778	0.812	4.458	0.027	0.271	0.659
R	0	1.180	0.967	11.822	0.030	0.589	0.640
	50	0.885	0.869	7.630	0.027	0.305	0.726
	100	0.812	0.816	7.555	0.027	0.262	0.720
	200	0.728	0.744	3.957	0.028	0.242	0.680
	400	0.659	0.677	2.382	0.028	0.196	0.730
	800	0.591	0.661	1.648	0.026	0.131	0.763

Table 2. Compressibility and fabric parameters: σ'_{pc} , Cr, Cc, $C_{c_{macro}}$, $C_{c_{micro}}$, $C_{uc_{macro}}$, $C_{uc_{micro}}$

Sample	Compressibility parameters			Fabric Parameters			
	σ'_{pc} (kPa)	Cr	Cc	$C_{c_{macro}}$	$C_{c_{micro}}$	$C_{uc_{macro}}$	$C_{uc_{micro}}$
I	40	0.004	0.662	0.339	0.061	0.344	-0.145
C	90	0.003	0.392	0.363	0.066	0.410	-0.215
R	-	0.002	0.253	0.155	0.076	0.216	-0.109

Figure captions

Figure 1. (a) Compaction Curve and study points; (b) Particle size distribution.

Figure 2. Oedometric compression curves.

Figure 3. MIP tests of intact samples: (a) cumulative curves, (b) PSD; compacted samples: (c) cumulative curves, (d) PSD; reconstituted samples: (e) cumulative curves, (f) PSD.

Figure 4. Compression curves of whole sample, macroporosity and microporosity: (a) Intact; (b) Compacted; (c) Reconstituted.

Figure 5. $C_{p_{macro}}$ and $C_{p_{micro}}$ determination in cumulative mercury intrusion curve of intact samples.

Figure 6. $C_{uc_{macro}}$, $C_{p_{macro}}$ and Cc: (a) Intact; (b) Compacted; (c) Reconstituted.

Figure 7. Relationship between $C_{uc_{macro}}$, $C_{c_{macro}}$ and Cc.

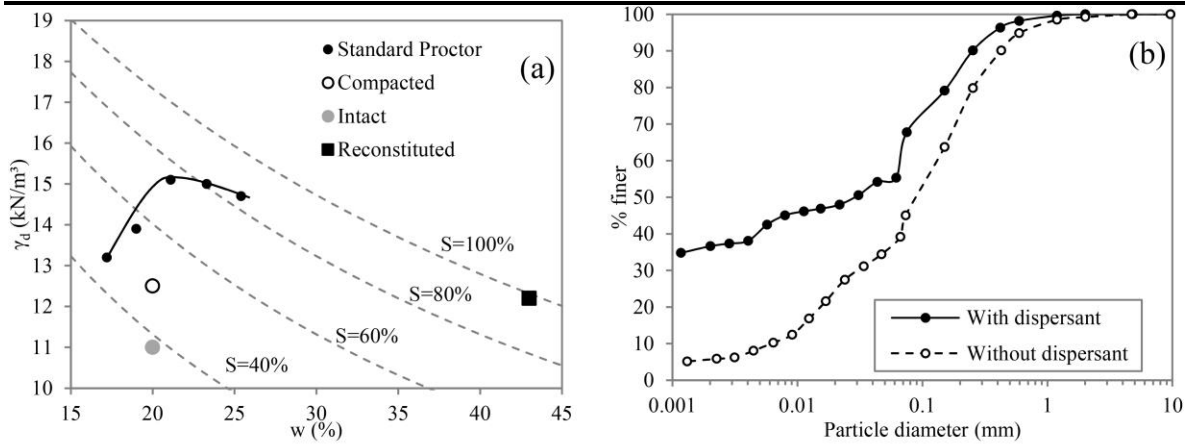


Figure 1

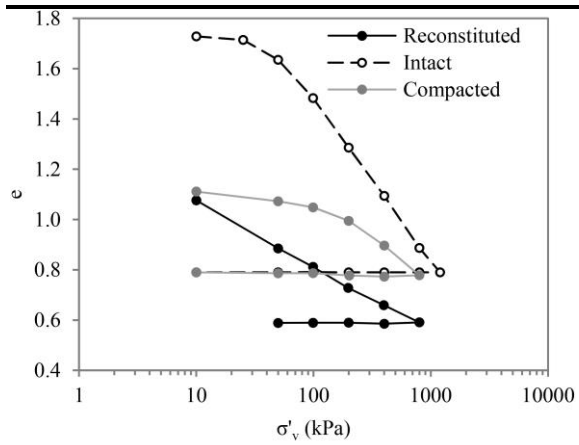


Figure 2

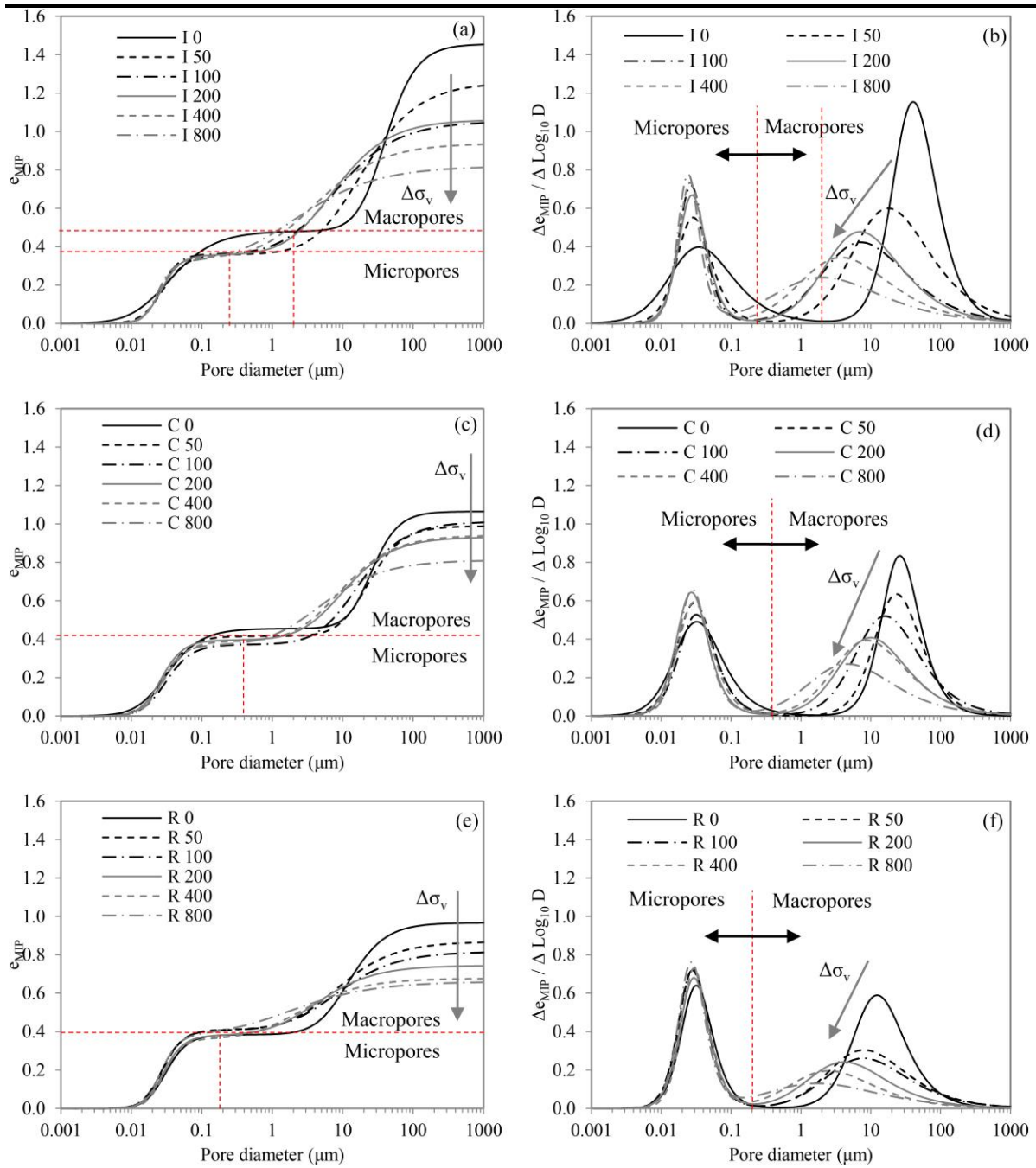


Figure 3

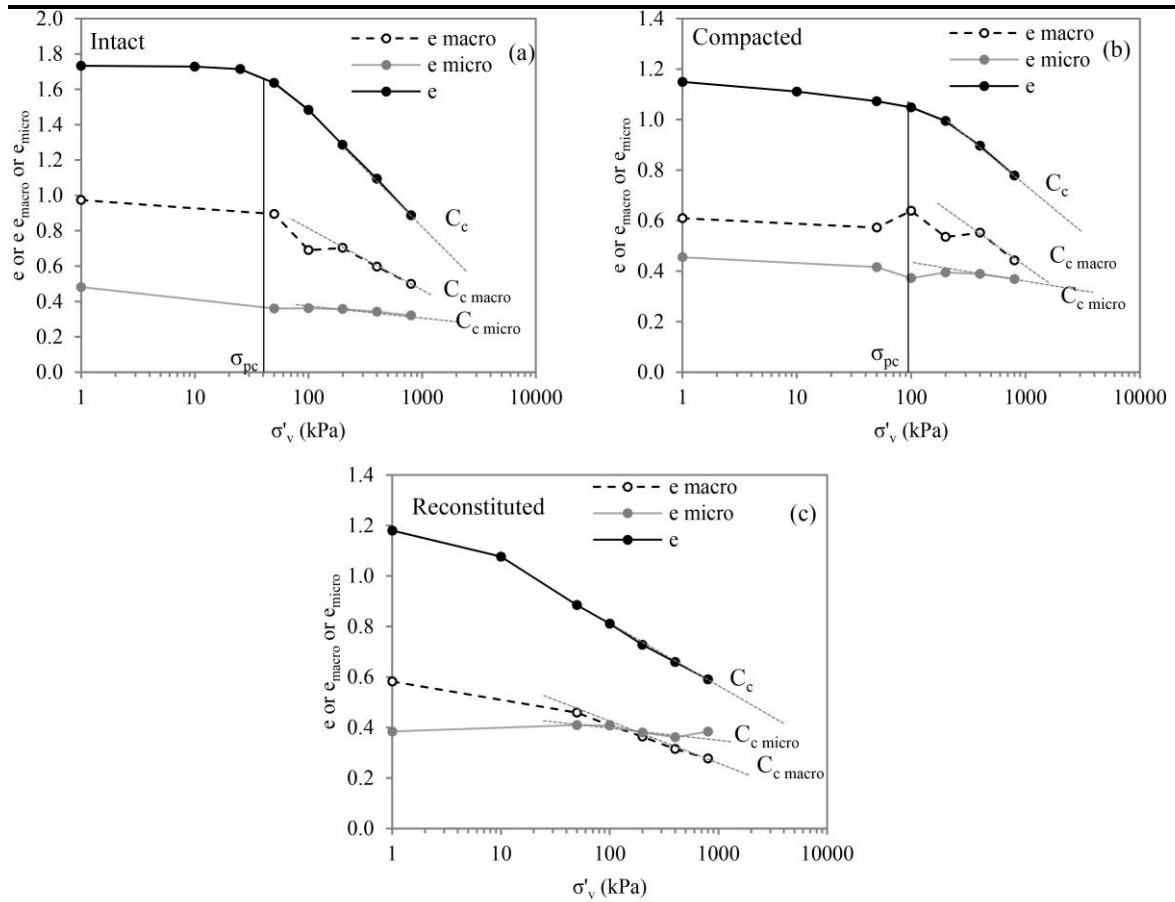


Figure 4

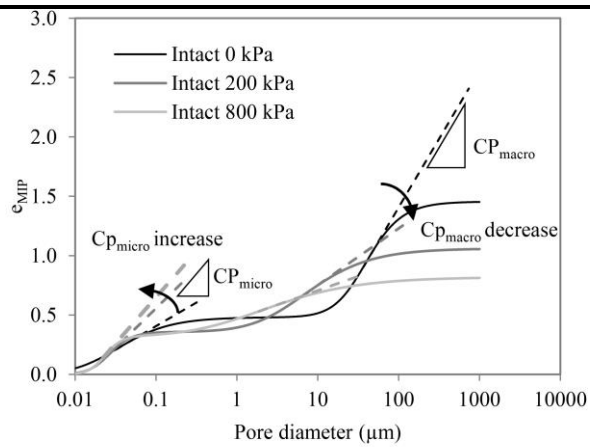


Figure 5

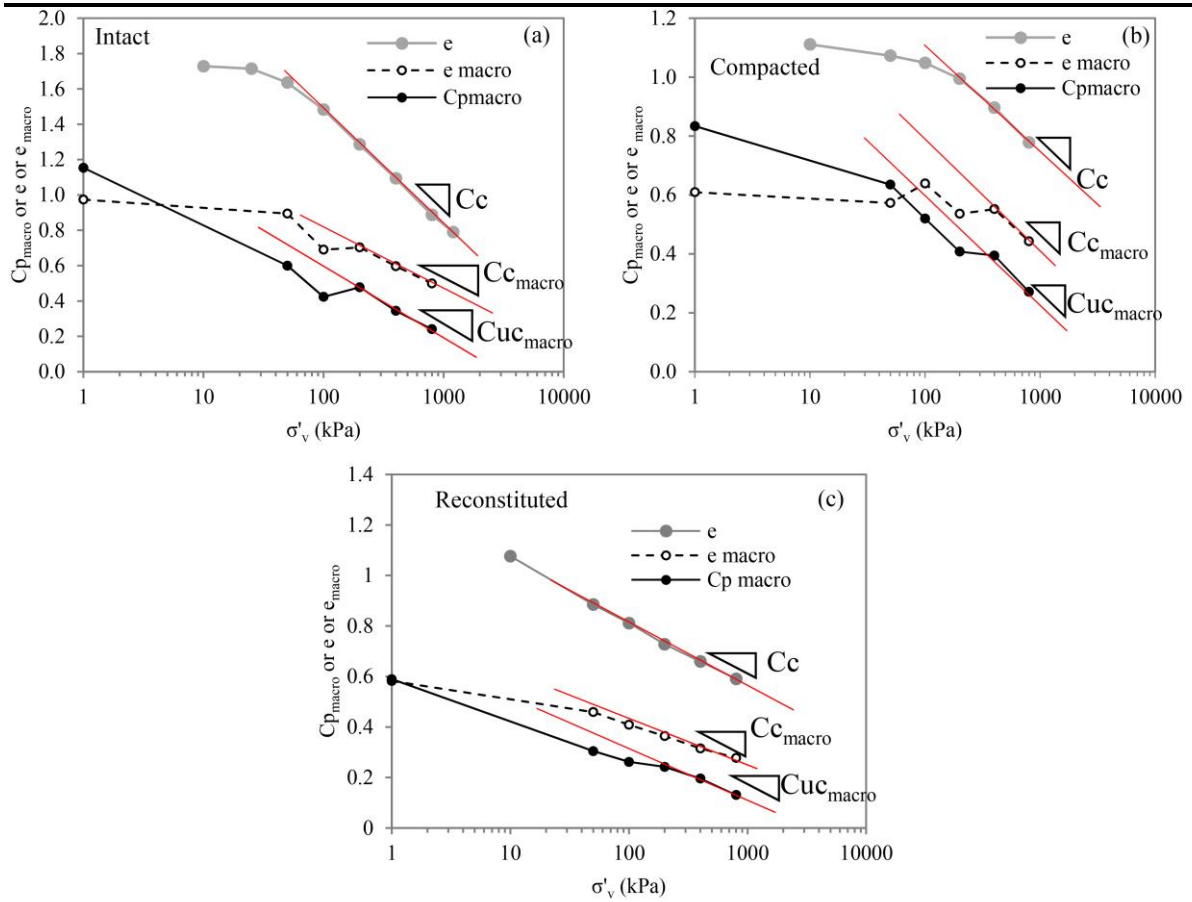


Figure 6

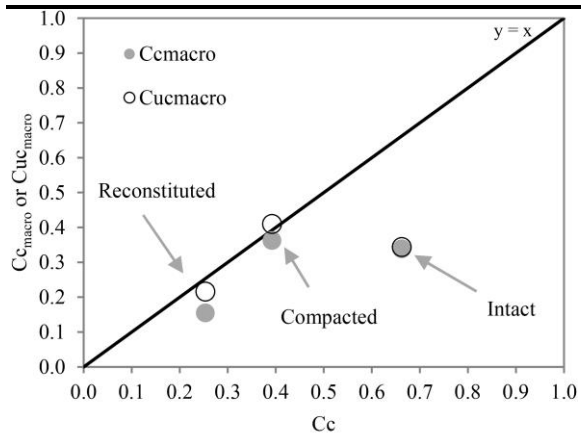


Figure 7.